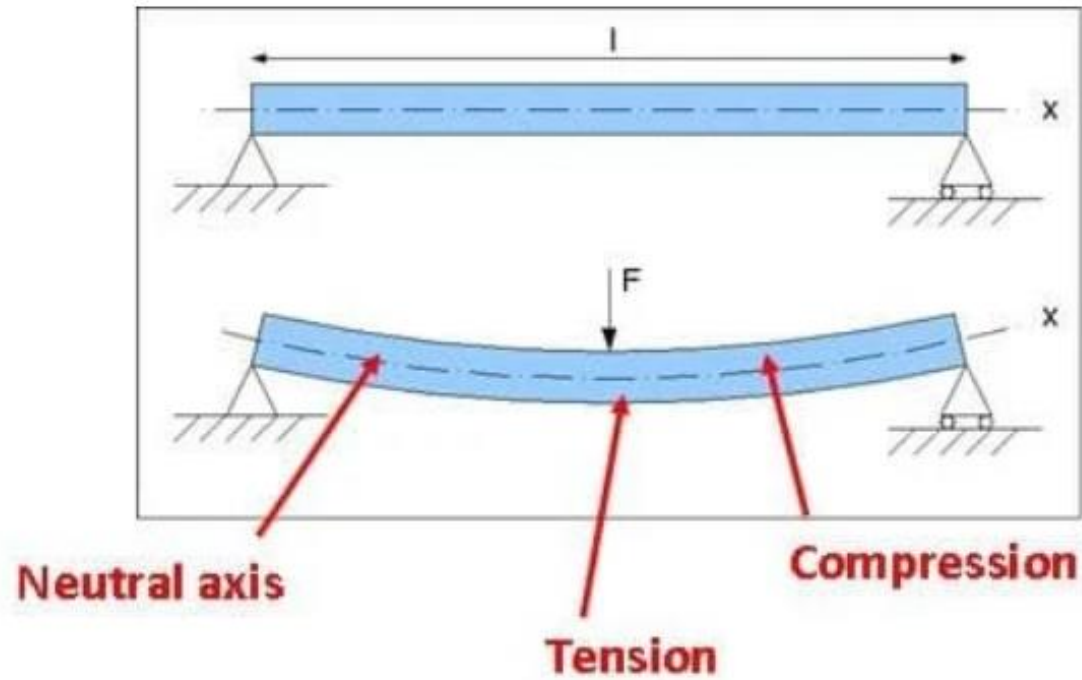


# فصل پنجم

## خمشی

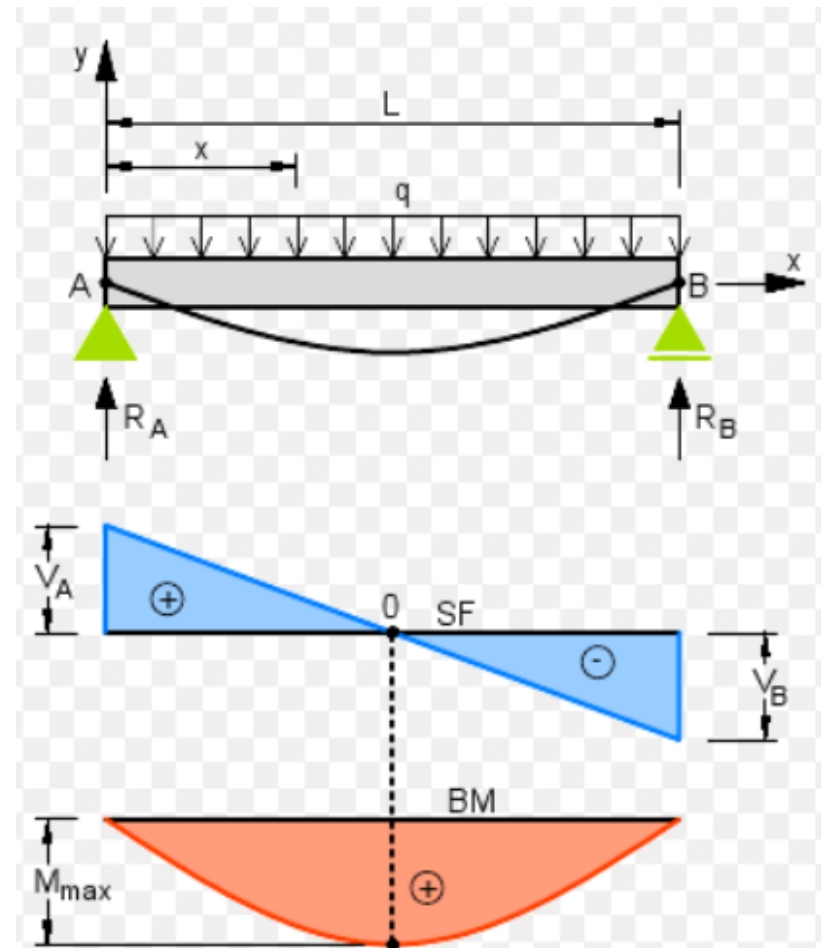
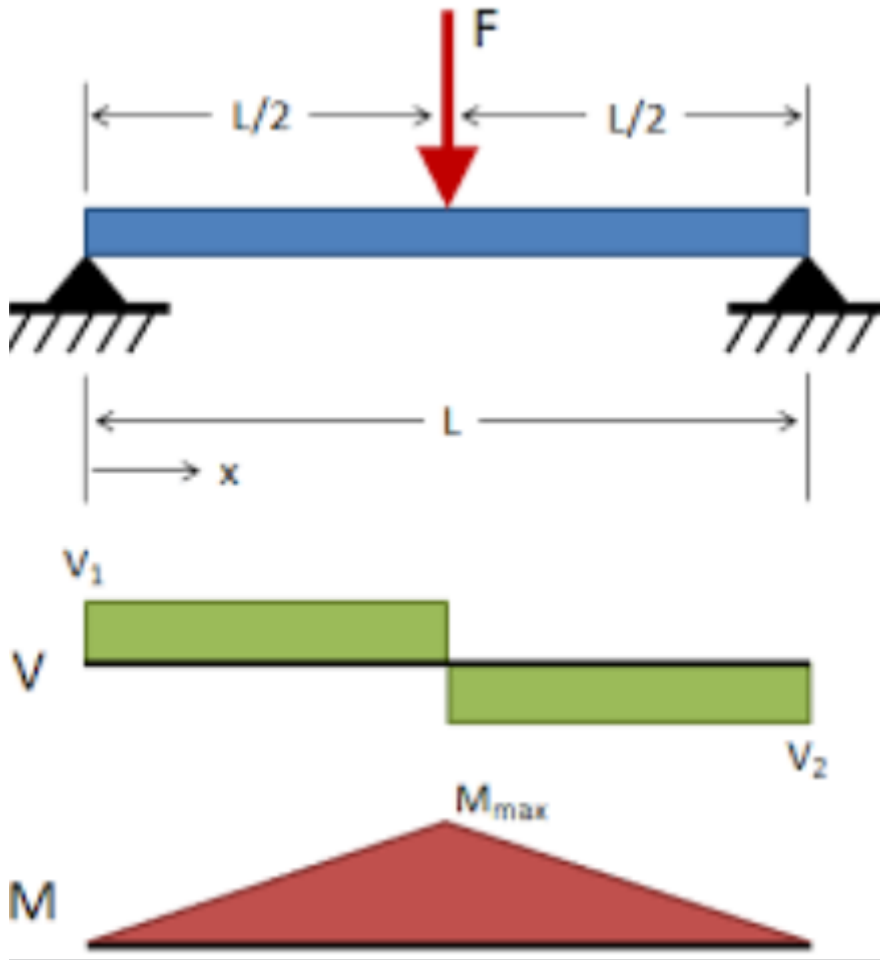
BENDING





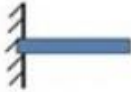

# BENDING

## فصل پنجم: خمشی

### SHEAR AND MOMENT DIAGRAMS

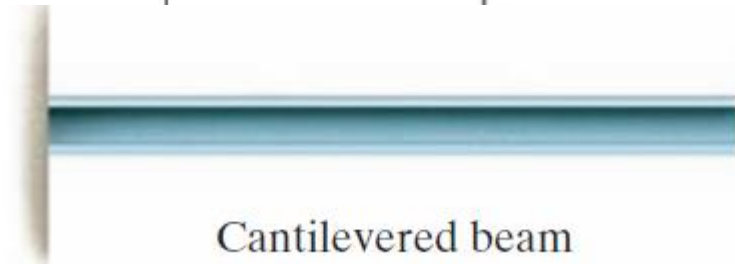


## SHEAR AND MOMENT DIAGRAMS

S.no	Types of Support	Representation by	Reaction Force	Resisting Load
1.	Roller Support		Vertical	Vertical loads
2.	Pinned Support		Horizontal and vertical	Vertical and horizontal loads
3.	Fixed Support		Horizontal, vertical and moments	All types of loads Horizontal, vertical and Moments
4.	Simple Support		Vertical	Vertical loads



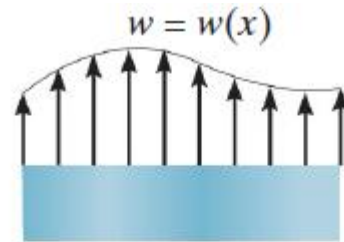
Simply supported beam



Cantilevered beam

## SHEAR AND MOMENT DIAGRAMS

### Beam Sign Convention.



Positive external distributed load



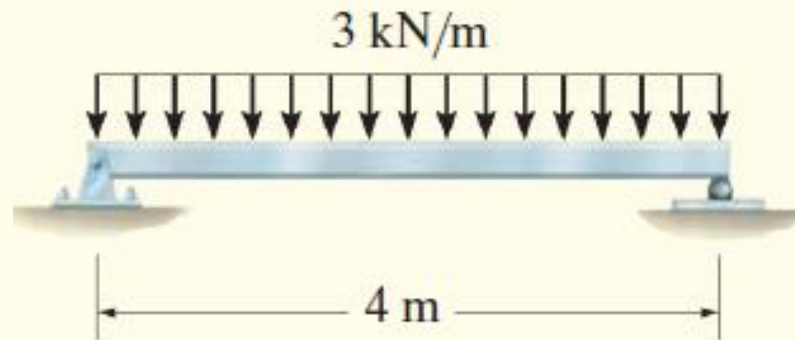
Positive internal shear

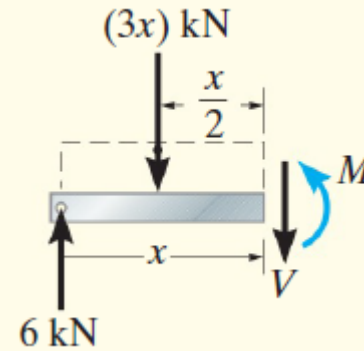
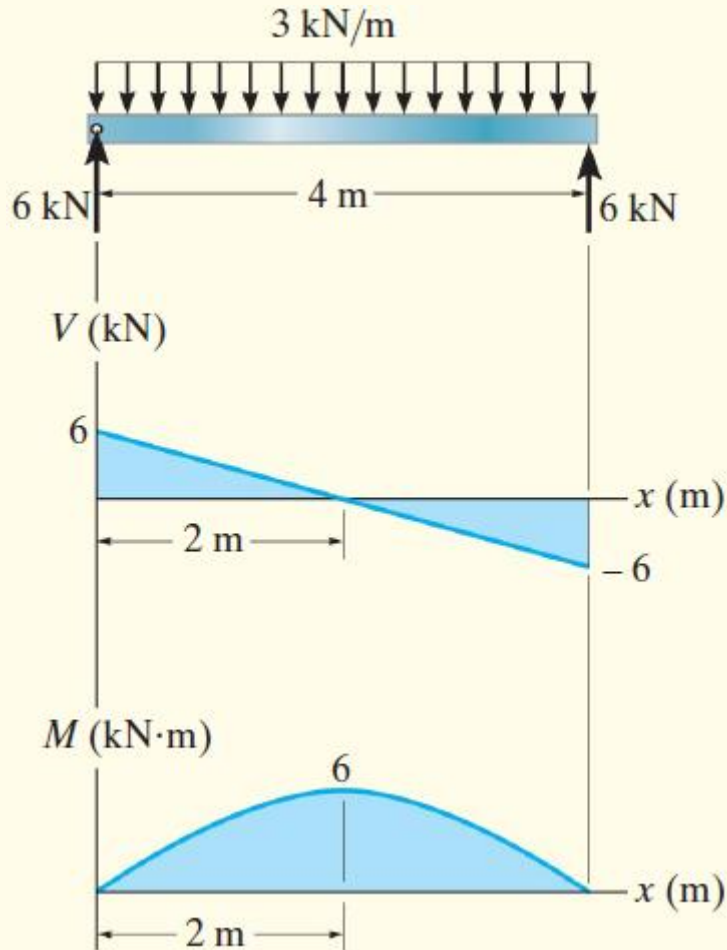


Positive internal moment

Beam sign convention

Draw the shear and moment diagrams for the beam shown in Fig.





$$+\uparrow \sum F_y = 0; \quad 6 \text{ kN} - (3x) \text{ kN} - V = 0$$

$$V = (6 - 3x) \text{ kN}$$

$$\zeta + \sum M = 0; \quad -6 \text{ kN}(x) + (3x) \text{ kN} \left(\frac{1}{2}x\right) + M = 0$$

$$M = (6x - 1.5x^2) \text{ kN} \cdot \text{m}$$

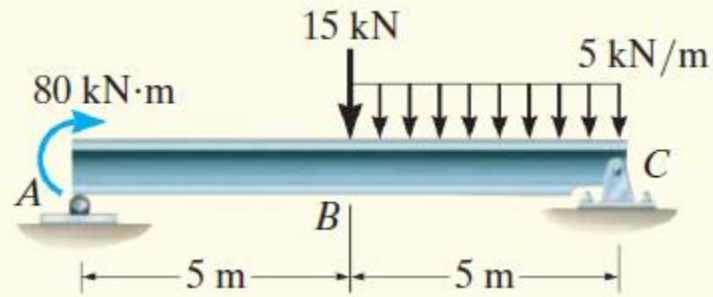
$$V = (6 - 3x) \text{ kN} = 0$$

$$x = 2 \text{ m}$$

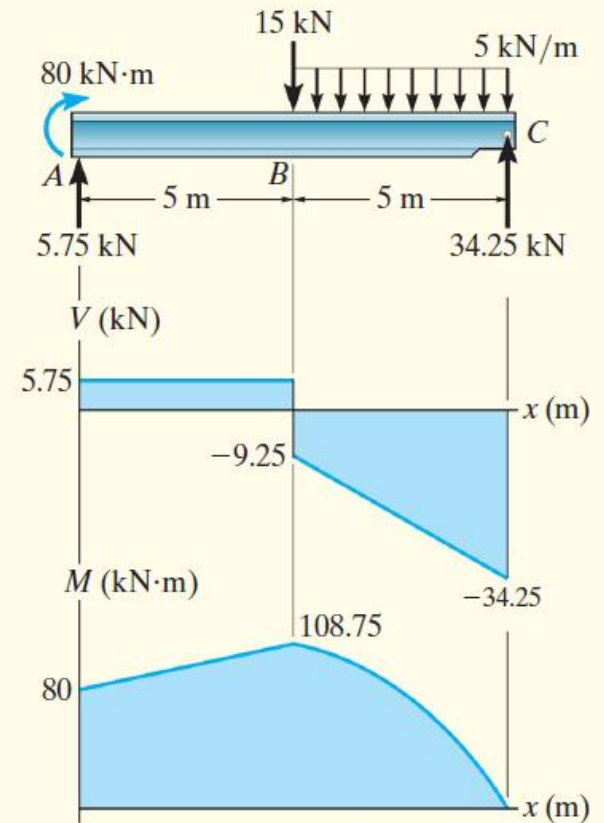
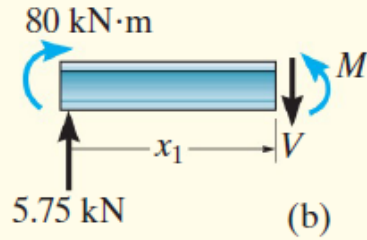
$$M_{\max} = [6(2) - 1.5(2)^2] \text{ kN} \cdot \text{m}$$

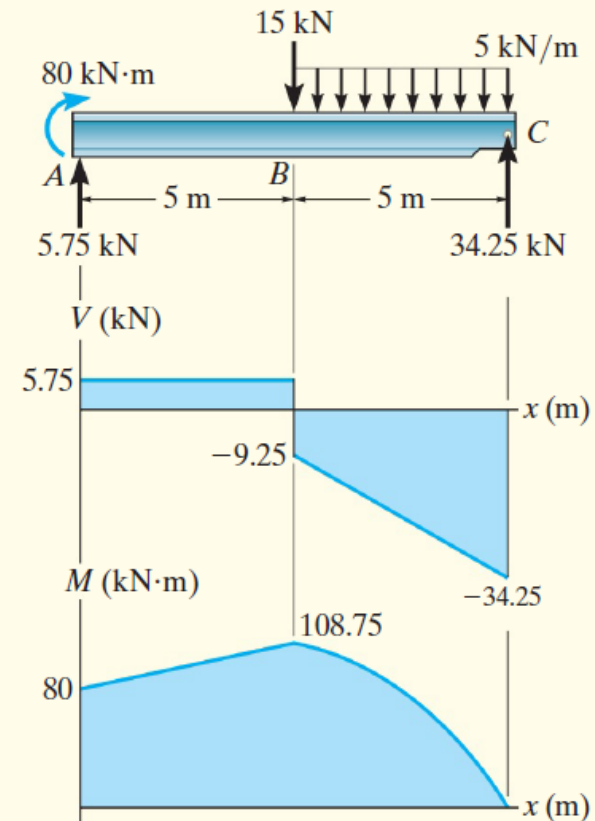
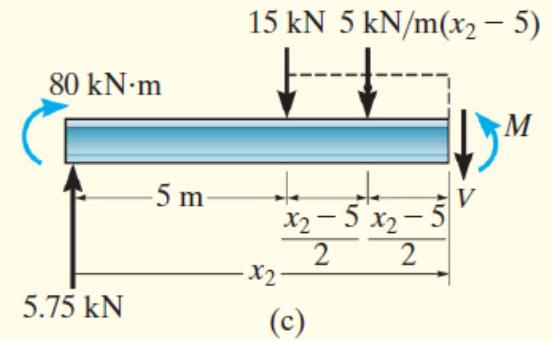
$$= 6 \text{ kN} \cdot \text{m}$$

Draw the shear and moment diagrams for the beam shown in Fig.

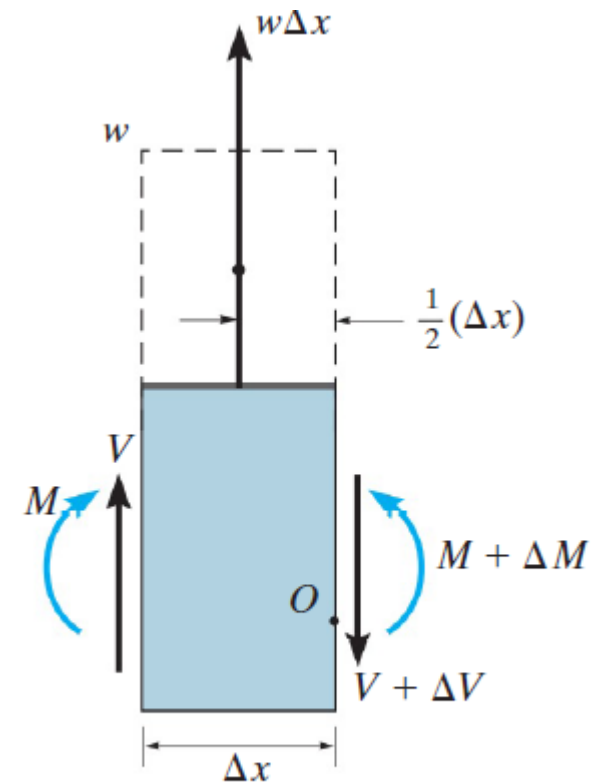
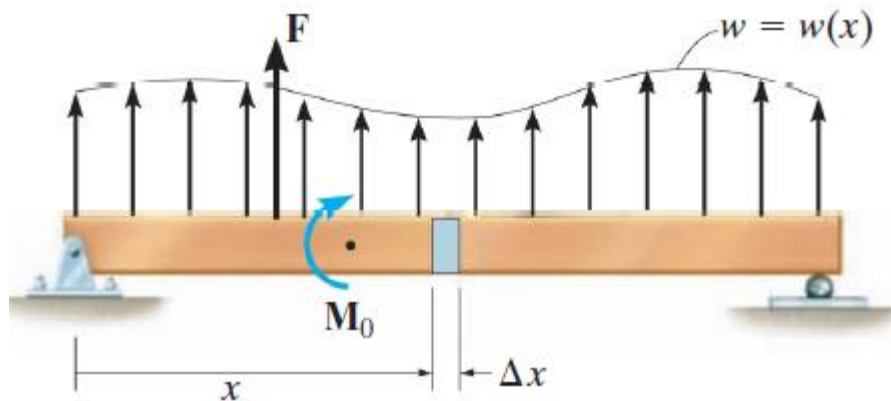








## GRAPHICAL METHOD FOR CONSTRUCTING SHEAR AND MOMENT DIAGRAMS



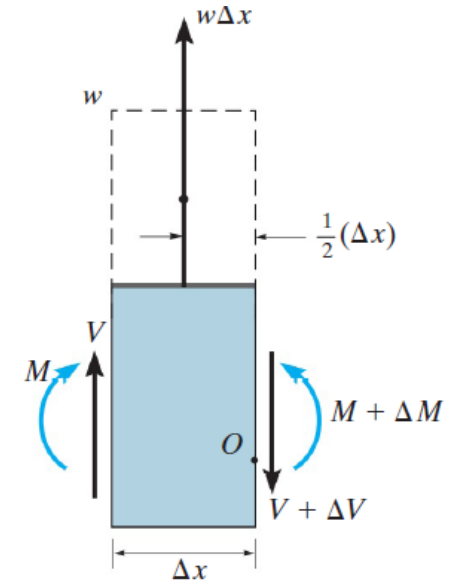
Free-body diagram of segment  $\Delta x$

$$+\uparrow \Sigma F_y = 0; \quad V + w \Delta x - (V + \Delta V) = 0$$

$$\Delta V = w \Delta x$$

$$\zeta + \Sigma M_O = 0; \quad -V \Delta x - M - w \Delta x \left[ \frac{1}{2}(\Delta x) \right] + (M + \Delta M) = 0$$

$$\Delta M = V \Delta x + w \frac{1}{2}(\Delta x)^2$$



Free-body diagram of segment  $\Delta x$

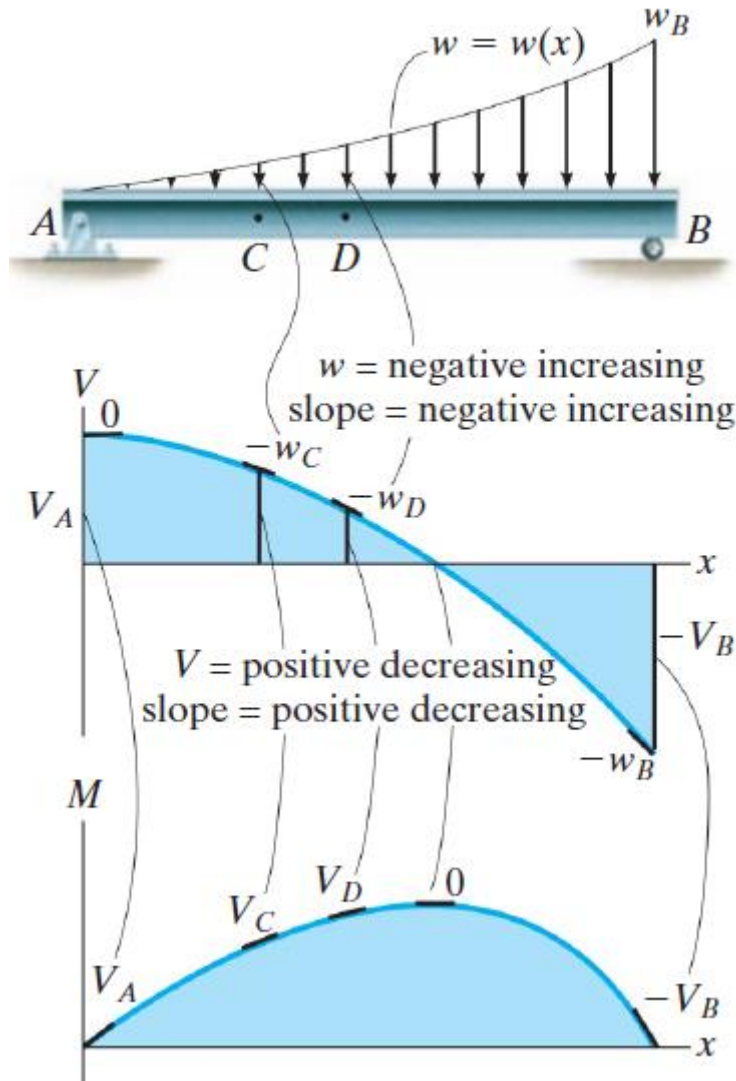
Dividing by  $\Delta x$  and taking the limit as  $\Delta x \rightarrow 0$ , the above two equations become

$$\frac{dV}{dx} = w$$

slope of shear diagram at each point = distributed load intensity at each point

$$\frac{dM}{dx} = V$$

slope of moment diagram at each point = shear at each point



$$\frac{dV}{dx} = w$$

slope of shear diagram at each point = distributed load intensity at each point

$$\frac{dM}{dx} = V$$

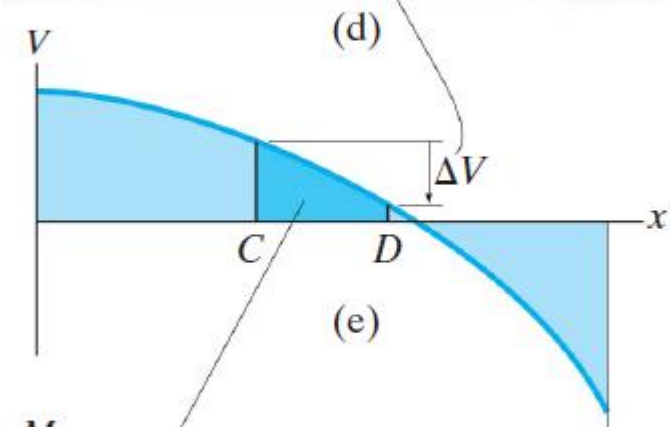
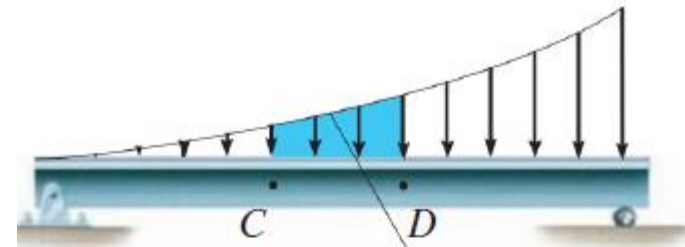
slope of moment diagram at each point = shear at each point

$$\frac{dV}{dx} = w$$

slope of shear diagram at each point      distributed load intensity at each point

$$\Delta V = \int w dx$$

change in shear      area under distributed loading

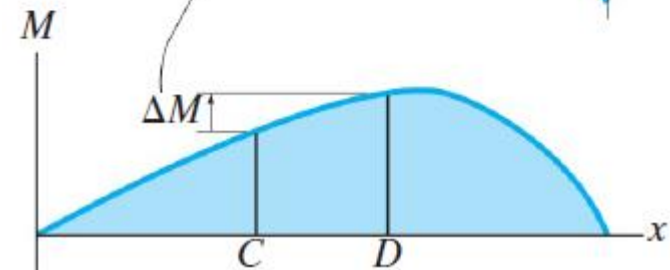


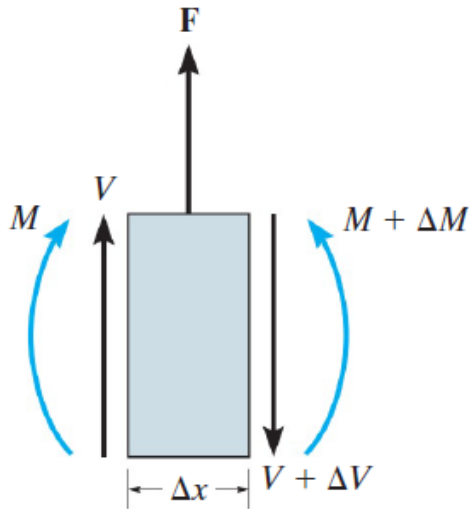
$$\frac{dM}{dx} = V$$

slope of moment diagram at each point      shear at each point

$$\Delta M = \int V dx$$

change in moment      area under shear diagram





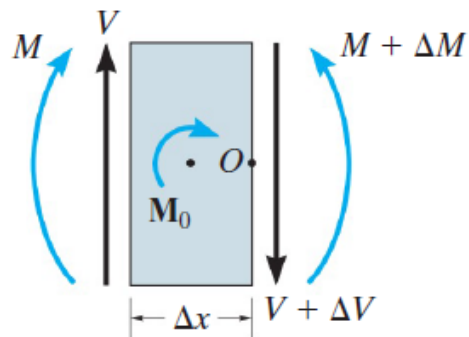
(a)

$$+\uparrow \Sigma F_y = 0;$$

$$V + F - (V + \Delta V) = 0$$

$$\Delta V = F$$

Thus, when  $\mathbf{F}$  acts *upward* on the beam, then the change in shear,  $V$ , is *positive* so the values of the shear on the shear diagram will “jump” *upward*.



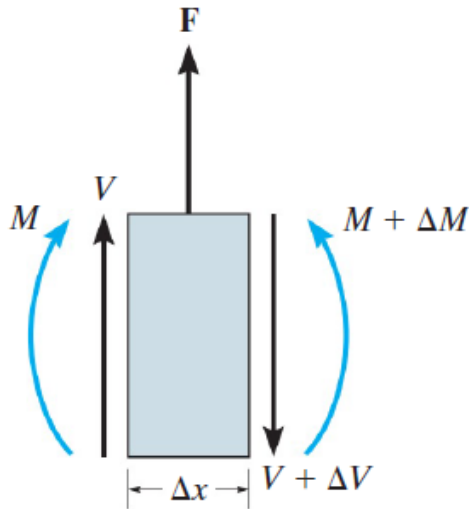
(b)

$$\zeta + \Sigma M_O = 0; \quad M + \Delta M - M_0 - V \Delta x - M = 0$$

Letting  $\Delta x \approx 0$ , we get

$$\Delta M = M_0$$

In this case, if  $\mathbf{M}_0$  is applied *clockwise*, the change in moment,  $M$ , is *positive* so the moment diagram will “jump” *upward*. Likewise, when  $\mathbf{M}_0$  acts *counterclockwise*, the jump ( $M$ ) will be *downward*.



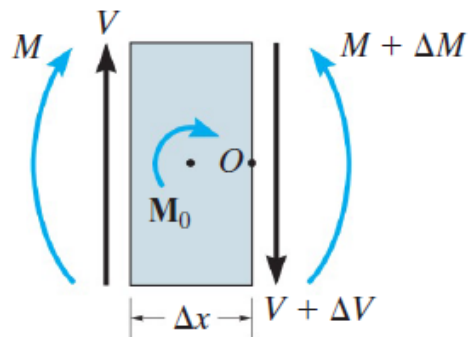
(a)

$$+\uparrow \Sigma F_y = 0;$$

$$V + F - (V + \Delta V) = 0$$

$$\Delta V = F$$

Thus, when  $\mathbf{F}$  acts *upward* on the beam, then the change in shear,  $V$ , is *positive* so the values of the shear on the shear diagram will “jump” *upward*.



(b)

$$\zeta + \Sigma M_O = 0; \quad M + \Delta M - M_0 - V \Delta x - M = 0$$

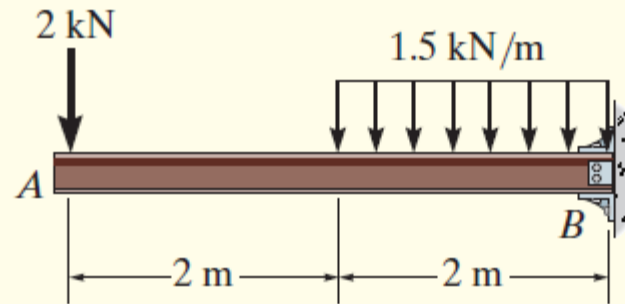
Letting  $\Delta x \approx 0$ , we get

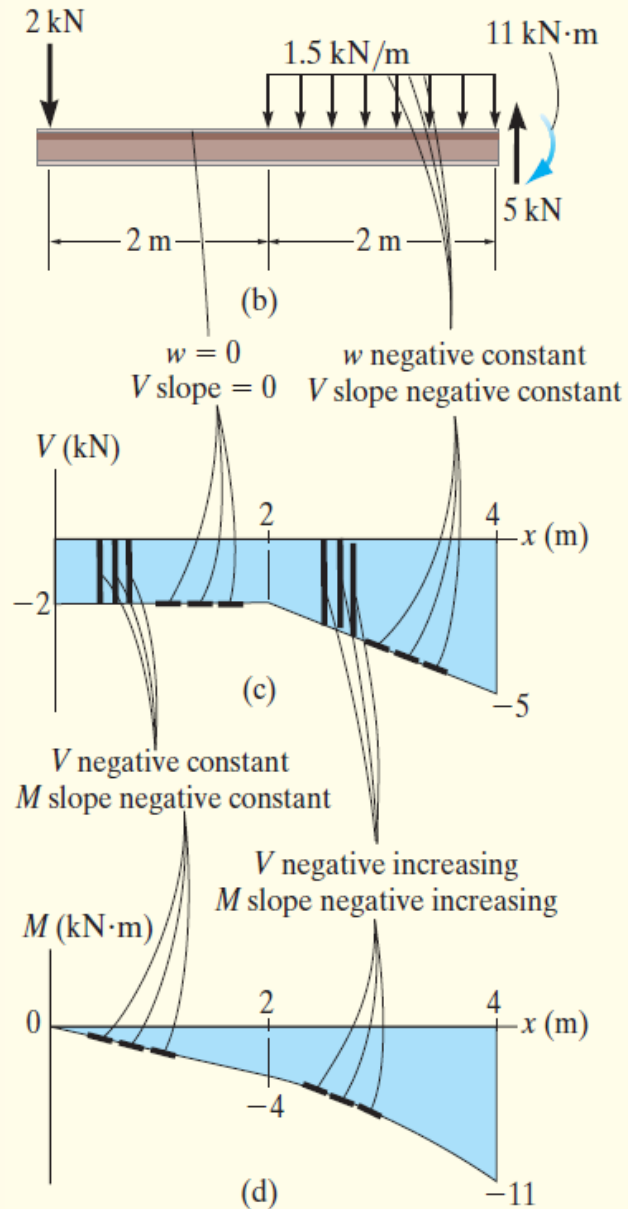
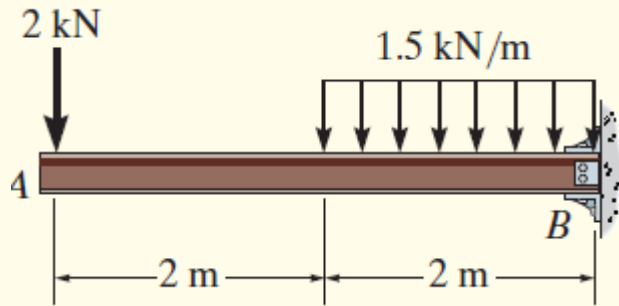
$$\Delta M = M_0$$

In this case, if  $\mathbf{M}_0$  is applied *clockwise*, the change in moment,  $M$ , is *positive* so the moment diagram will “jump” *upward*. Likewise, when  $\mathbf{M}_0$  acts *counterclockwise*, the jump ( $M$ ) will be *downward*.

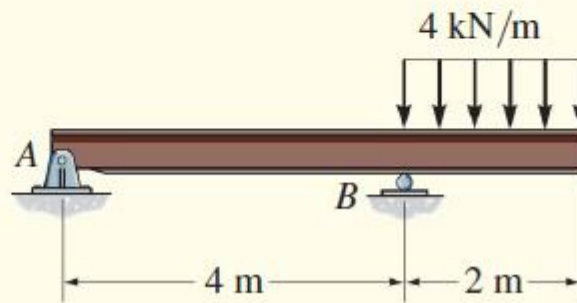


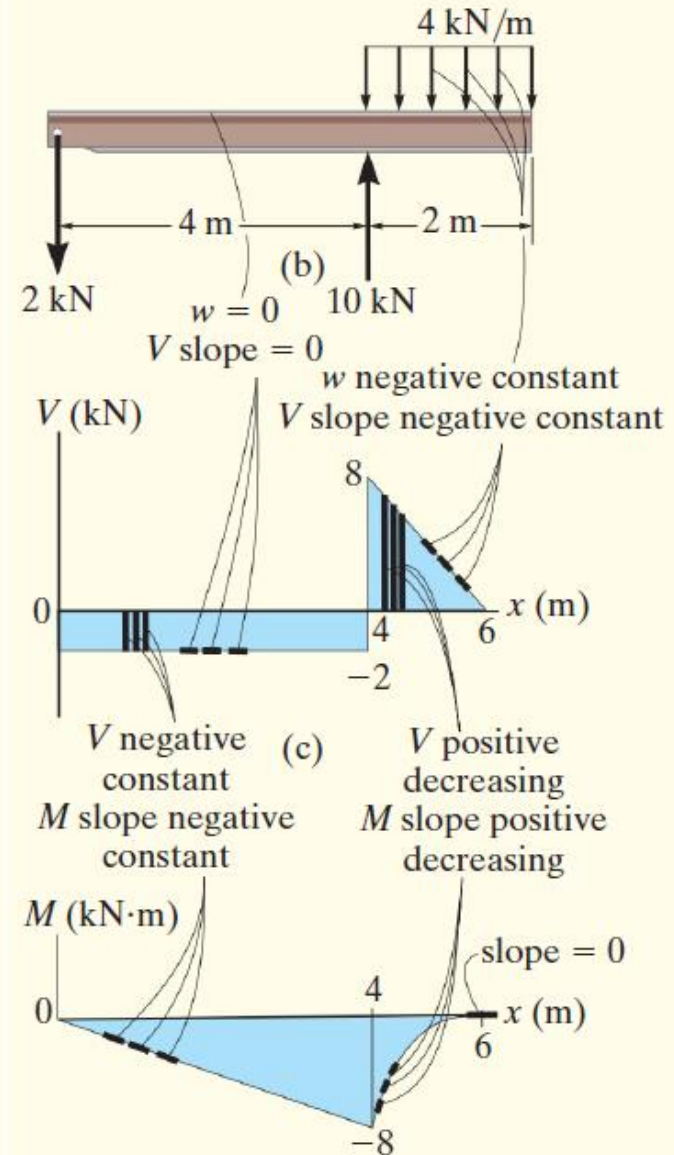
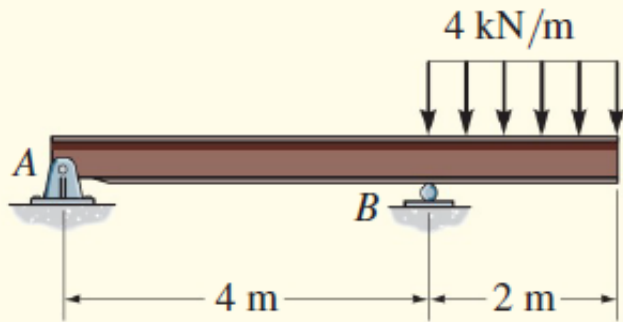
Draw the shear and moment diagrams for the cantilever beam in Fig.



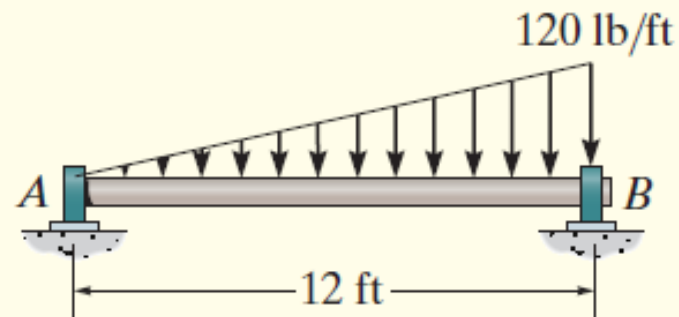


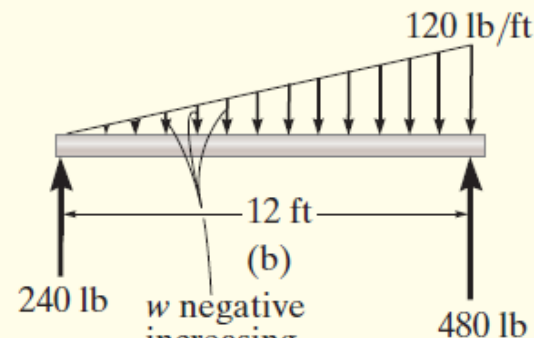
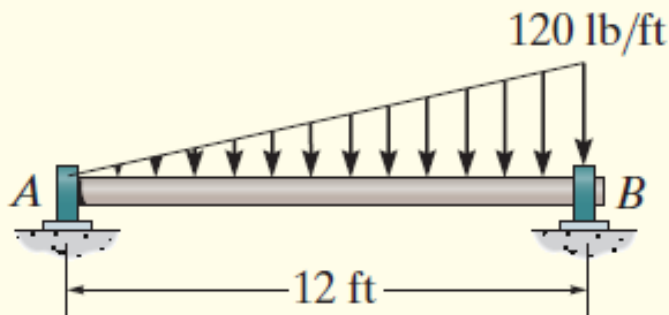
Draw the shear and moment diagrams for the overhang beam in Fig.





The shaft in Fig. 6–17a is supported by a thrust bearing at  $A$  and a journal bearing at  $B$ . Draw the shear and moment diagrams.



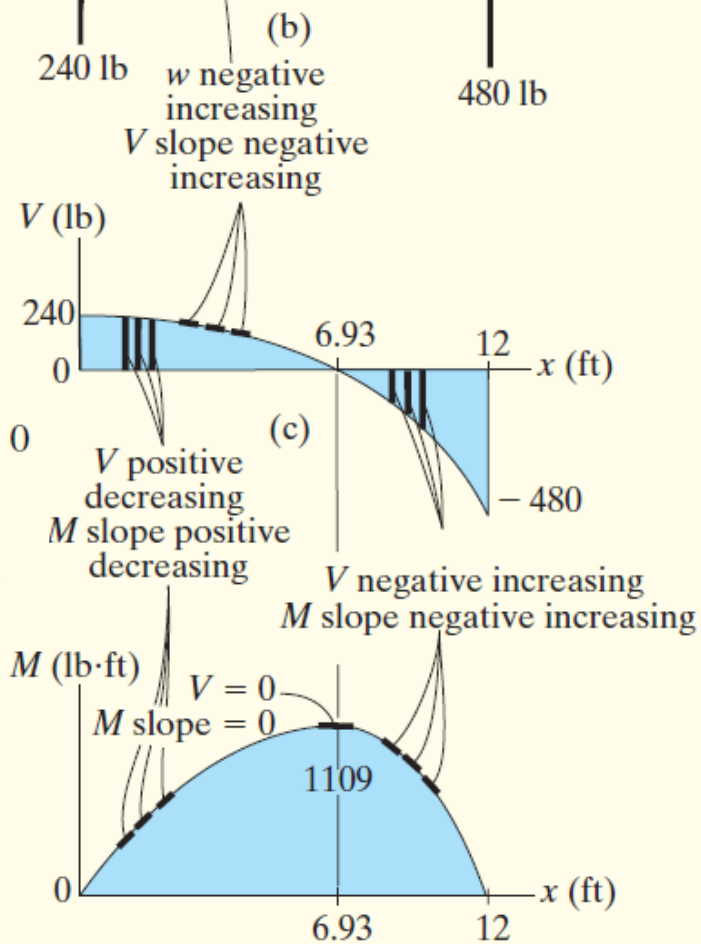
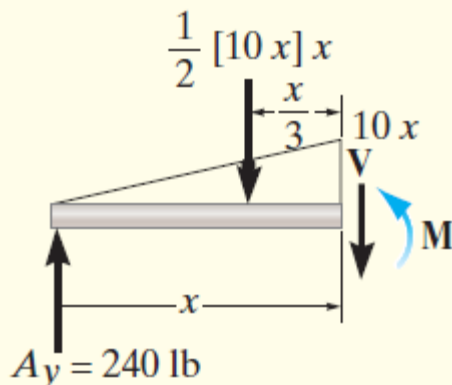


$$+\uparrow \sum F_y = 0; \quad 240 \text{ lb} - \frac{1}{2}(10x)x = 0$$

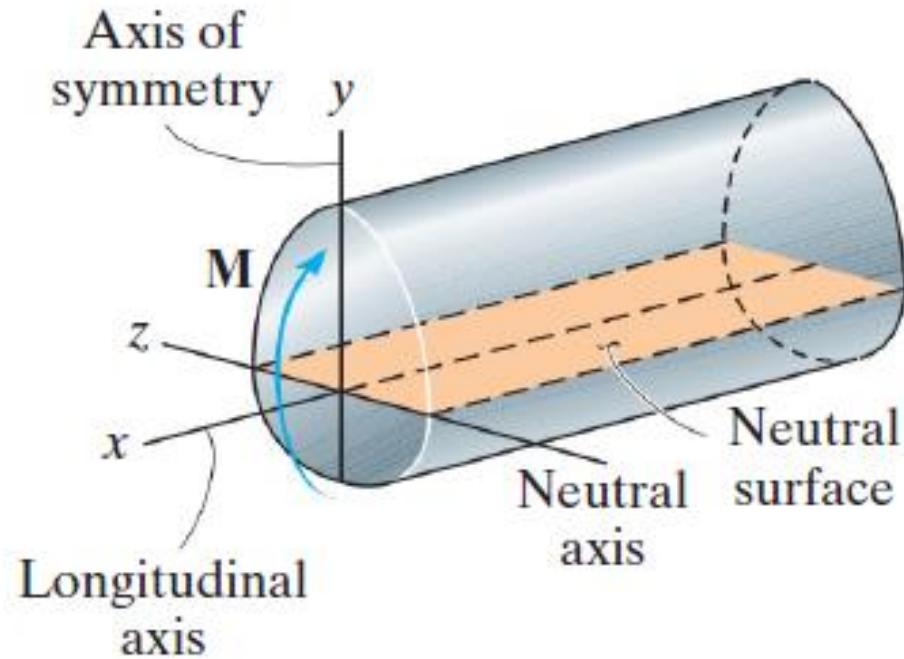
$$x = 6.93 \text{ ft}$$

$$\zeta + \sum M = 0; \quad M_{\max} + \frac{1}{2}[(10)(6.93)] 6.93 \left(\frac{1}{3}(6.93)\right) - 240(6.93) = 0$$

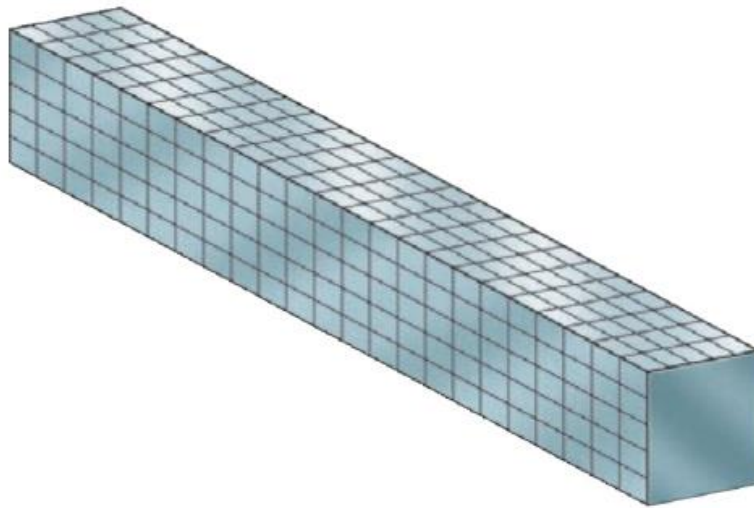
$$M_{\max} = 1109 \text{ lb} \cdot \text{ft}$$



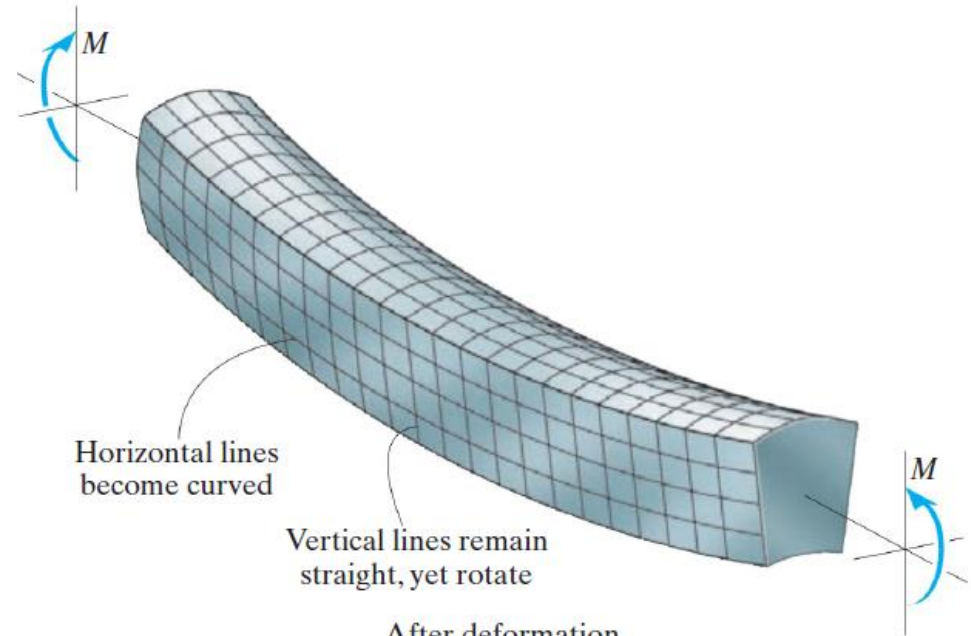
## BENDING DEFORMATION OF A STRAIGHT MEMBER



## BENDING DEFORMATION OF A STRAIGHT MEMBER

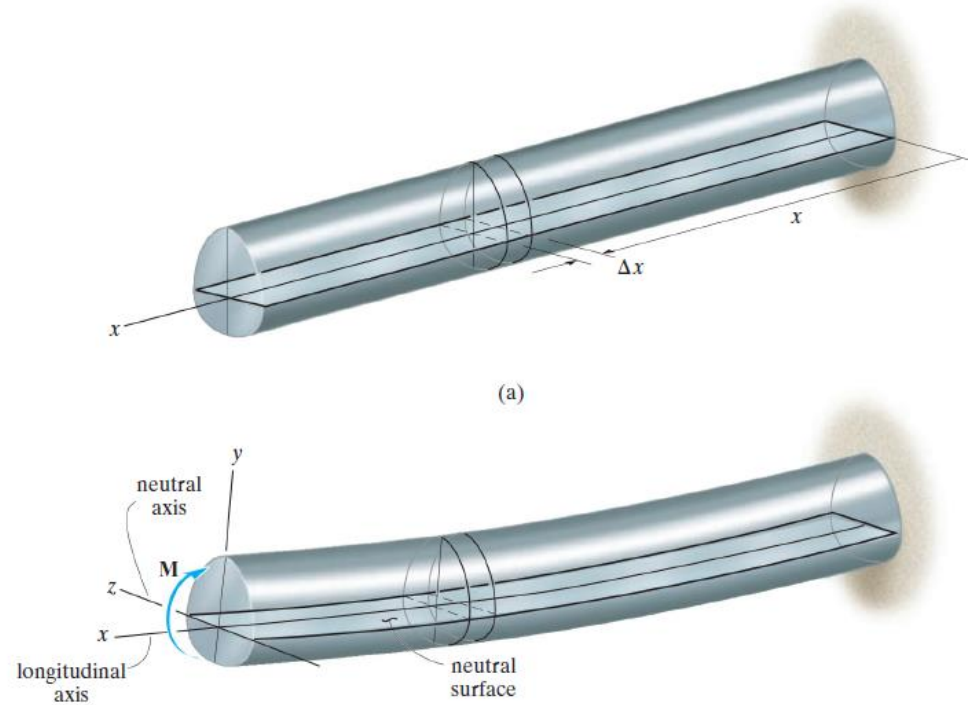
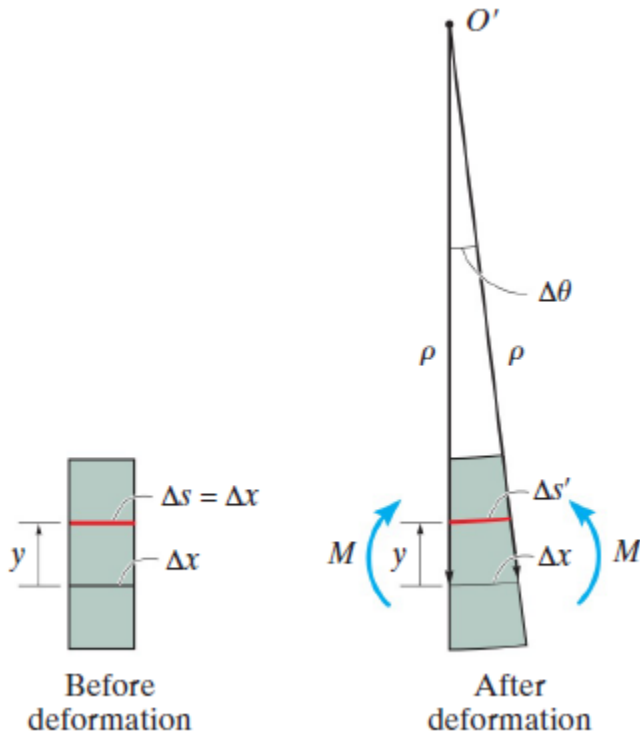


Before deformation



After deformation





$$\epsilon = \lim_{\Delta s \rightarrow 0} \frac{\Delta s' - \Delta s}{\Delta s}$$

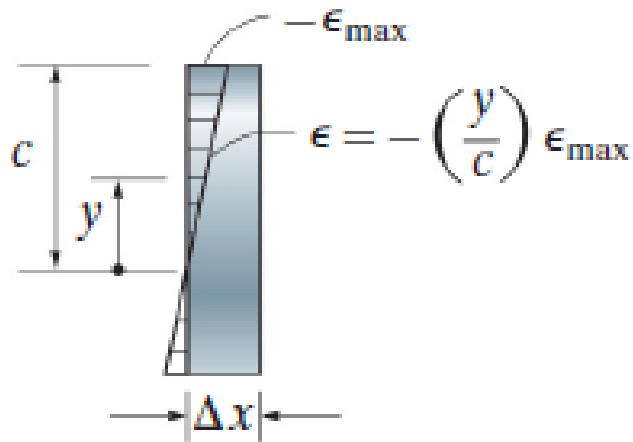
$$\Delta x = \Delta s = \rho \Delta \theta.$$

$$\Delta s' = (\rho - y) \Delta \theta.$$

$$\epsilon = \lim_{\Delta \theta \rightarrow 0} \frac{(\rho - y) \Delta \theta - \rho \Delta \theta}{\rho \Delta \theta}$$

$$\epsilon = -\frac{y}{\rho}$$

$$\epsilon = -\frac{y}{\rho}$$



Normal strain distribution

$$\frac{\epsilon}{\epsilon_{\max}} = -\left(\frac{y/\rho}{c/\rho}\right)$$

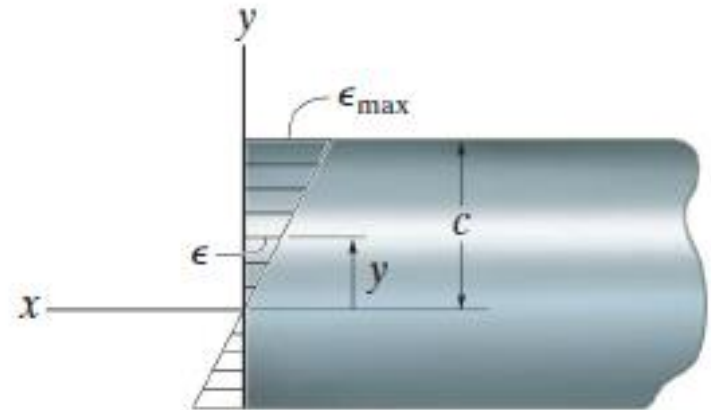
$$\epsilon = -\left(\frac{y}{c}\right)\epsilon_{\max}$$

## THE FLEXURE FORMULA

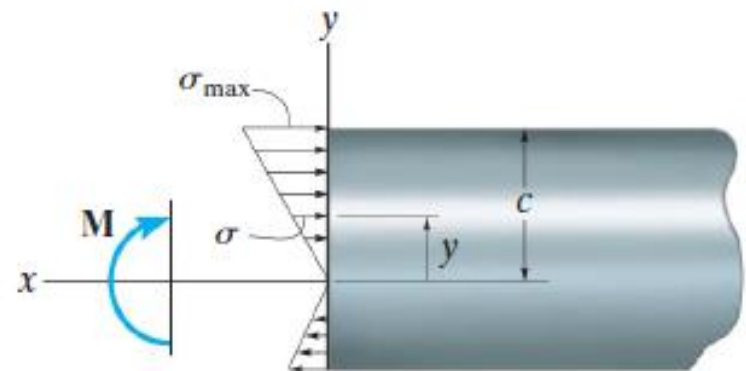
$$\sigma = E\epsilon$$

$$\epsilon = -\left(\frac{y}{c}\right)\epsilon_{\max}$$

$$\sigma = -\left(\frac{y}{c}\right)\sigma_{\max}$$



Normal strain variation  
(profile view)



Bending stress variation  
(profile view)

## Location of Neutral Axis.

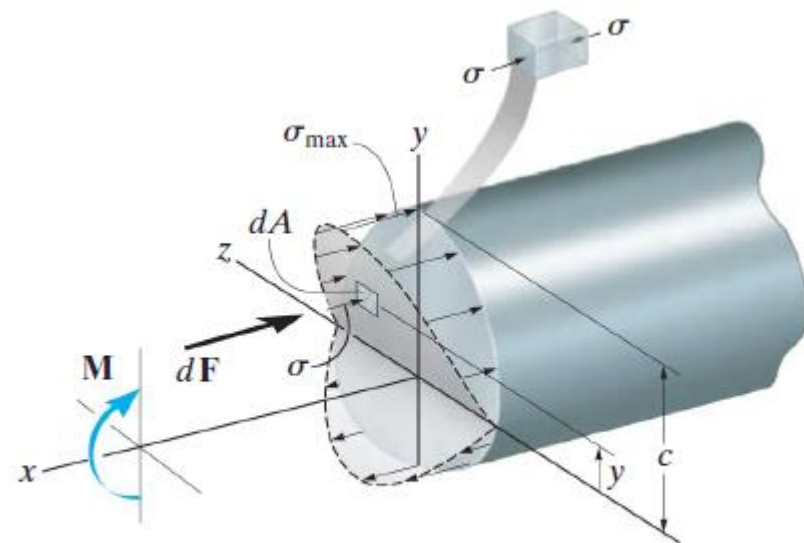
$$\begin{aligned}F_R = \Sigma F_x; \quad 0 &= \int_A dF = \int_A \sigma dA \\ &= \int_A -\left(\frac{y}{c}\right)\sigma_{\max} dA \\ &= \frac{-\sigma_{\max}}{c} \int_A y dA\end{aligned}$$

Since  $\sigma_{\max}/c$  is not equal to zero, then

$$\int_A y dA = 0$$

$$\bar{y} = \int y dA / \int dA.$$

$$\bar{y} = 0,$$

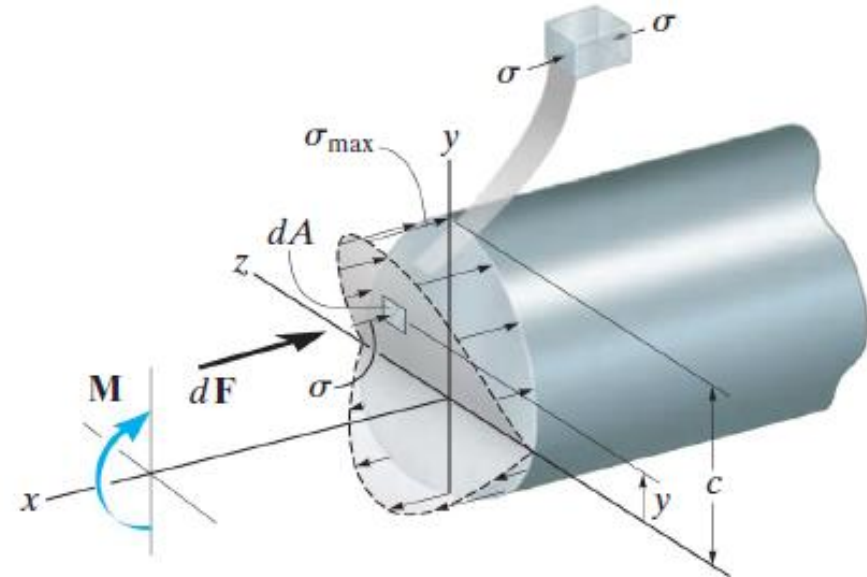


Bending stress variation

## Bending Moment.

$$dM = y dF.$$

$$dF = \sigma dA$$



Bending stress variation

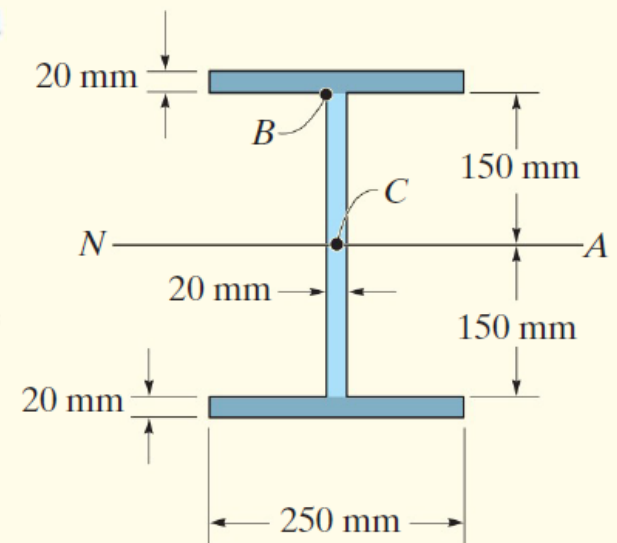
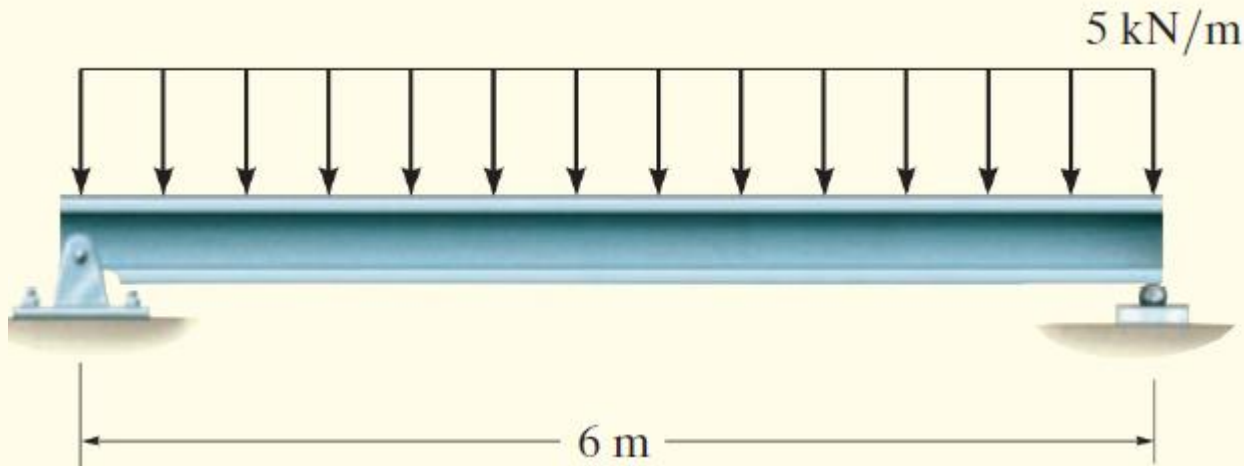
$$(M_R)_z = \Sigma M_z; \quad M = \int_A y dF = \int_A y (\sigma dA) = \int_A y \left( \frac{y}{c} \sigma_{\max} \right) dA$$

$$M = \frac{\sigma_{\max}}{c} \int_A y^2 dA$$

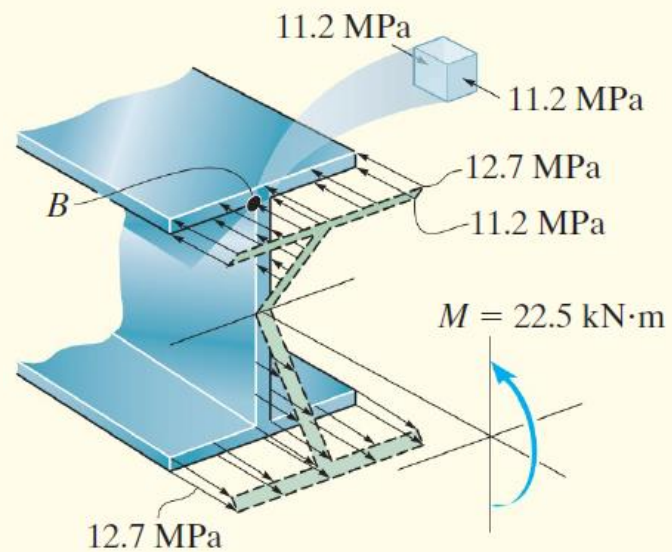
$$\sigma_{\max} = \frac{Mc}{I}$$

$$\sigma = - \frac{My}{I}$$

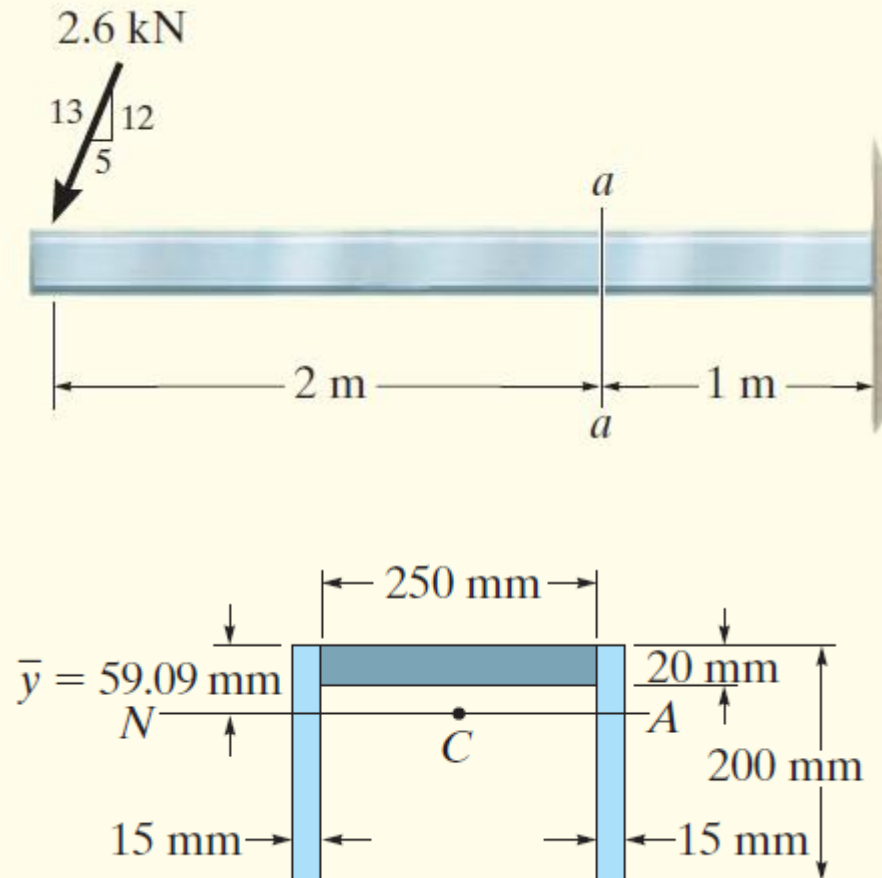
The simply supported beam in Fig. 6–26*a* has the cross-sectional area shown in Fig. 6–26*b*. Determine the absolute maximum bending stress in the beam and draw the stress distribution over the cross section at this location. Also, what is the stress at point *B*?



## Section Property.

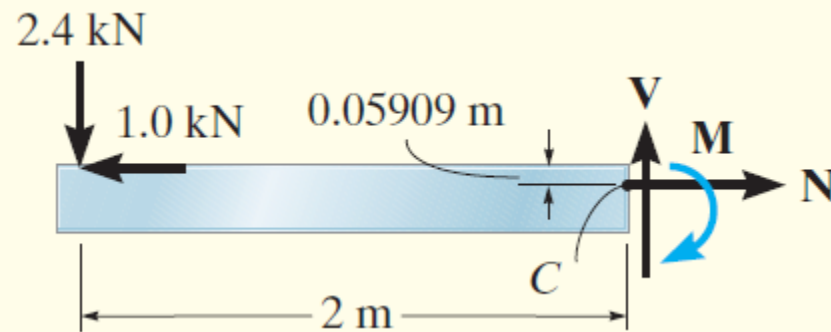
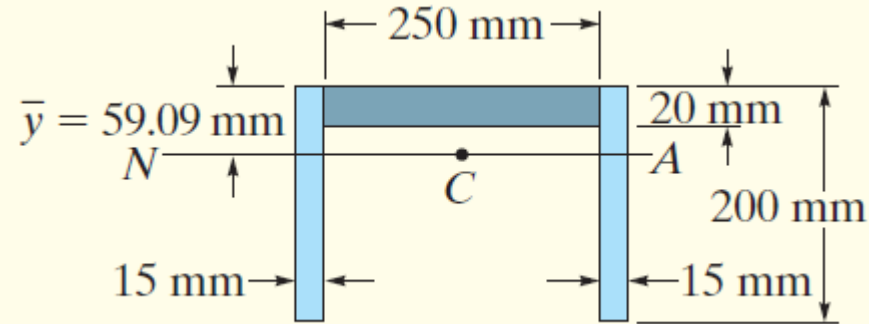


The beam shown in Fig. 6-27a has a cross-sectional area in the shape of a channel, Fig. 6-27b. Determine the maximum bending stress that occurs in the beam at section  $a-a$ .

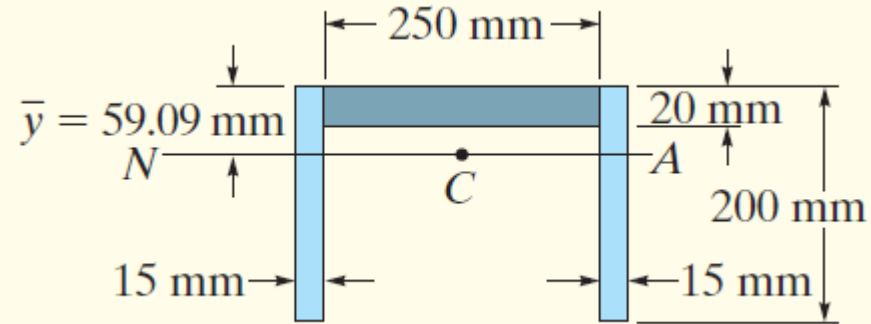


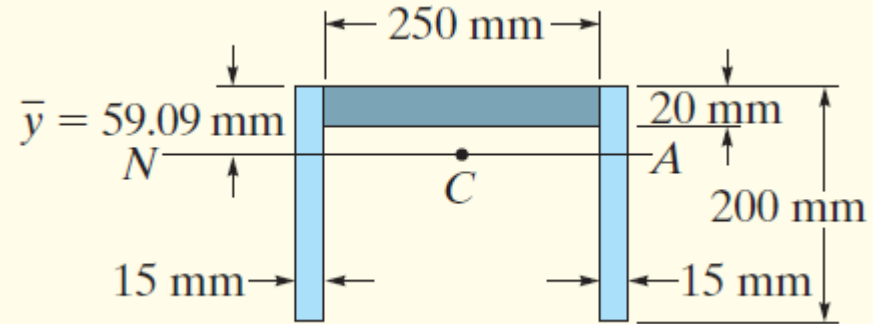


## Internal Moment.



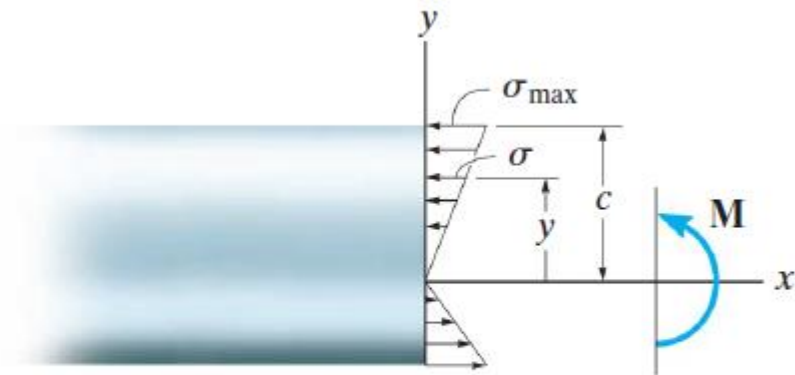
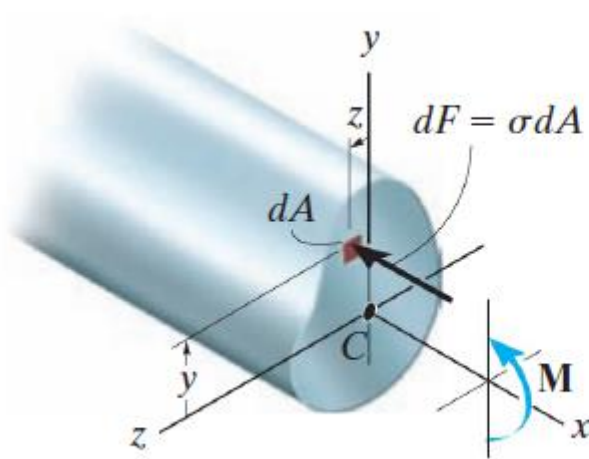
### Section Property.





## UNSYMMETRIC BENDING

### Moment Applied About Principal Axis.



Bending-stress distribution  
(profile view)

$$F_R = \Sigma F_x;$$

$$(M_R)_y = \Sigma M_y;$$

$$(M_R)_z = \Sigma M_z;$$

$$0 = - \int_A \sigma dA$$

$$0 = - \int_A z\sigma dA$$

$$M = \int_A y\sigma dA$$

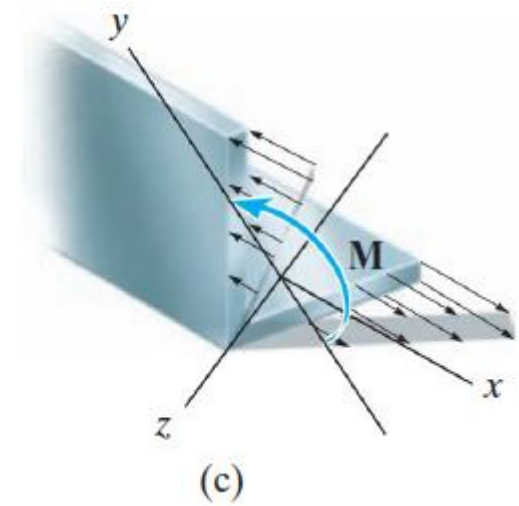
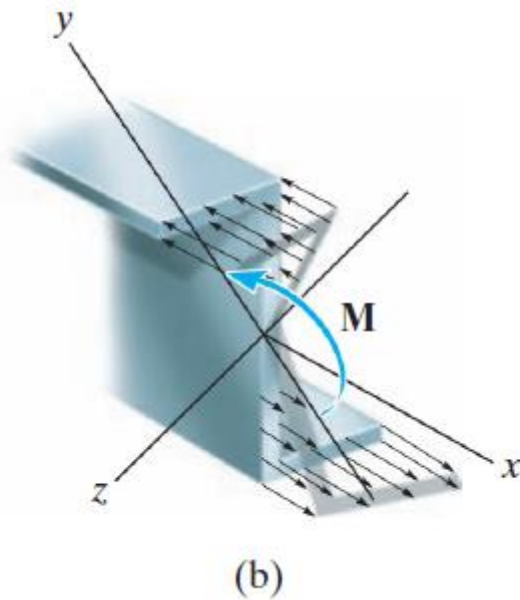
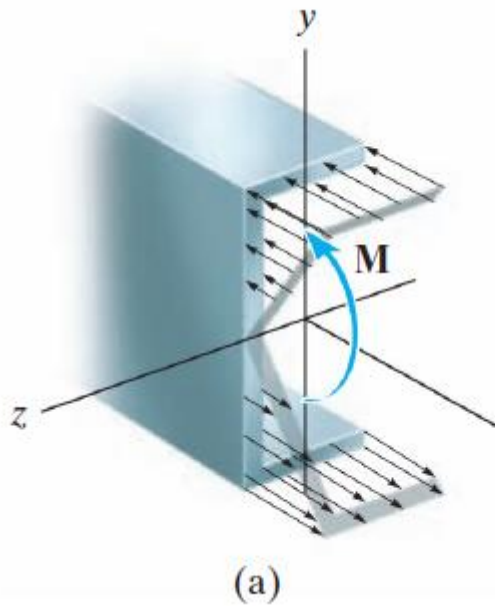
### UNSYMMETRIC BENDING

$$\sigma = -(y/c)\sigma_{\max} \quad M = \int_A y\sigma dA \quad \longrightarrow \quad \sigma_{\max} = Mc/I.$$

$$\sigma = -(y/c)\sigma_{\max} \quad 0 = -\int_A z\sigma dA \quad \longrightarrow \quad 0 = \frac{-\sigma_{\max}}{c} \int_A yz dA$$

$$\int_A yz dA = 0 \quad \text{product of inertia}$$

## UNSYMMETRIC BENDING



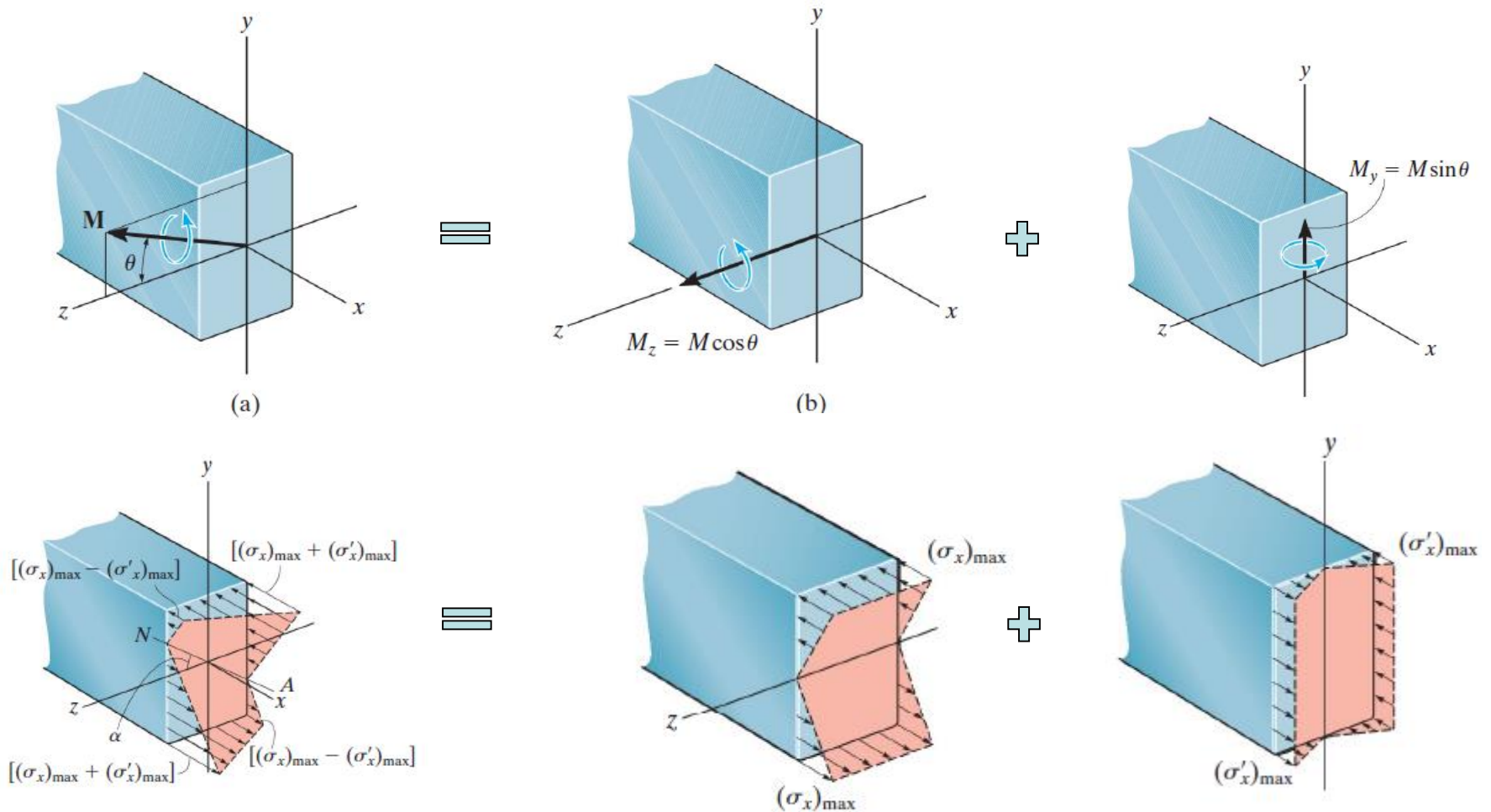
$$\sigma = -My/I_z$$

# BENDING

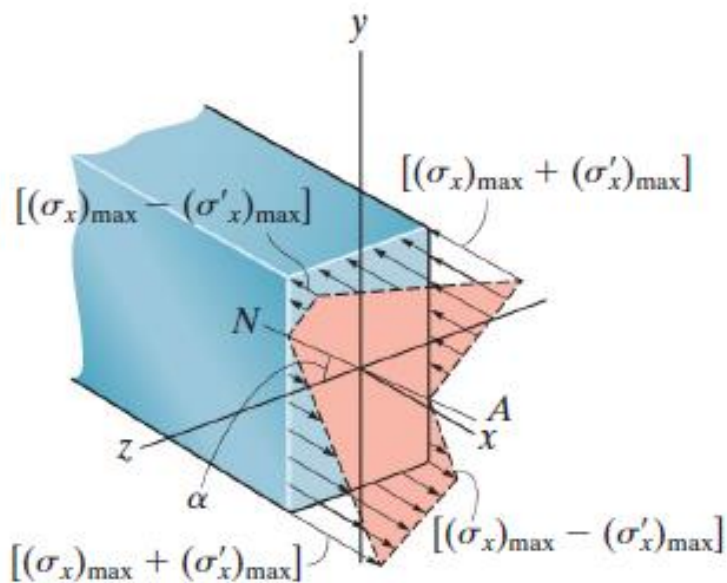
## فصل پنجم: خمشی

### UNSYMMETRIC BENDING

#### Moment Arbitrarily Applied.



## UNSYMMETRIC BENDING Moment Arbitrarily Applied.



$$\sigma = -\frac{M_z y}{I_z} + \frac{M_y z}{I_y}$$

### Orientation of the Neutral Axis.

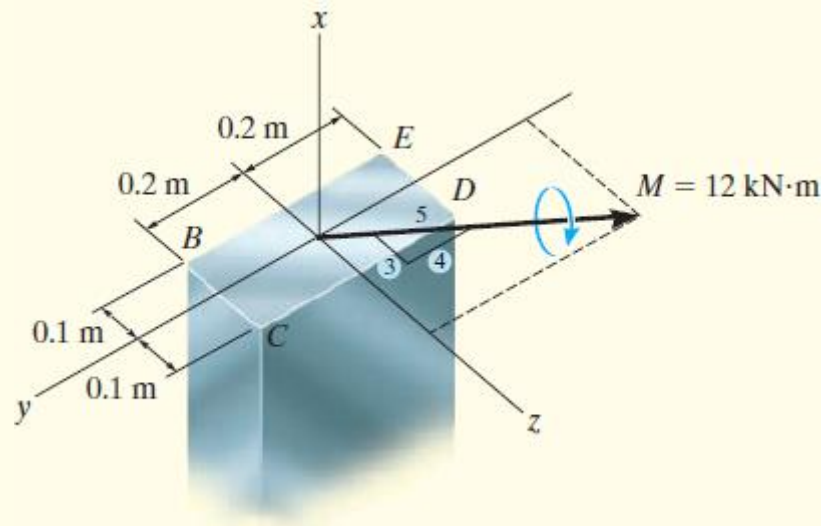
$$\sigma = 0, \quad y = \frac{M_y I_z}{M_z I_y} z$$

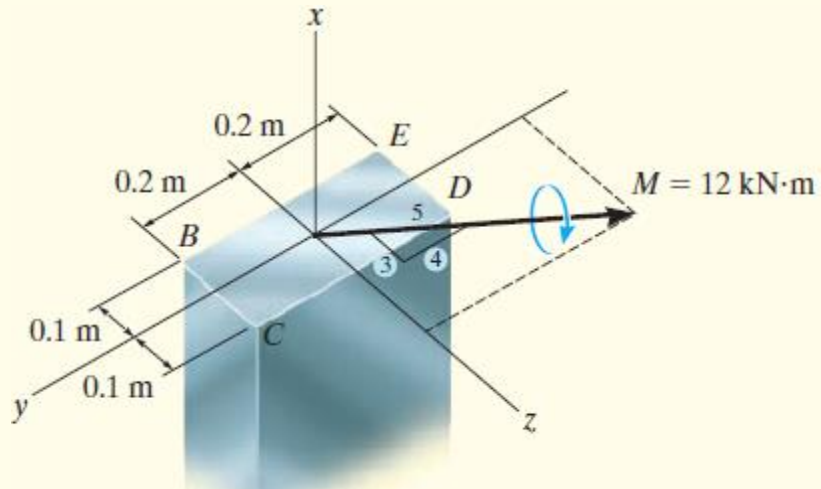
Since  $M_z = M \cos \theta$  and  $M_y = M \sin \theta$ , then

$$y = \left( \frac{I_z}{I_y} \tan \theta \right) z$$



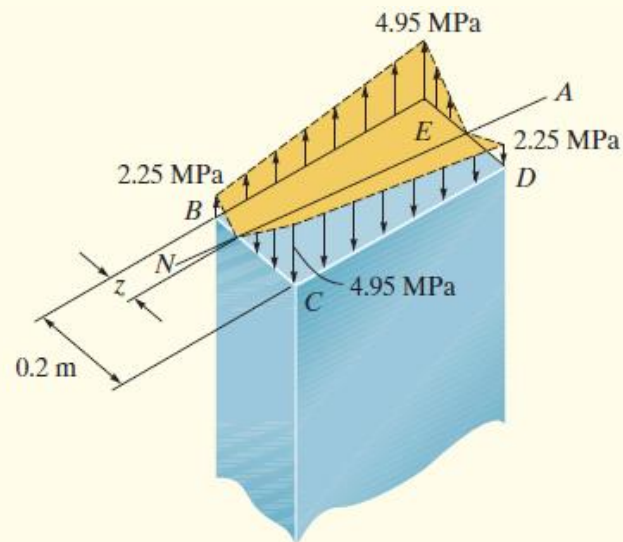
The rectangular cross section shown in Fig. 6–33a is subjected to a bending moment of  $M = 12 \text{ kN} \cdot \text{m}$ . Determine the normal stress developed at each corner of the section, and specify the orientation of the neutral axis.





Internal Moment Components.

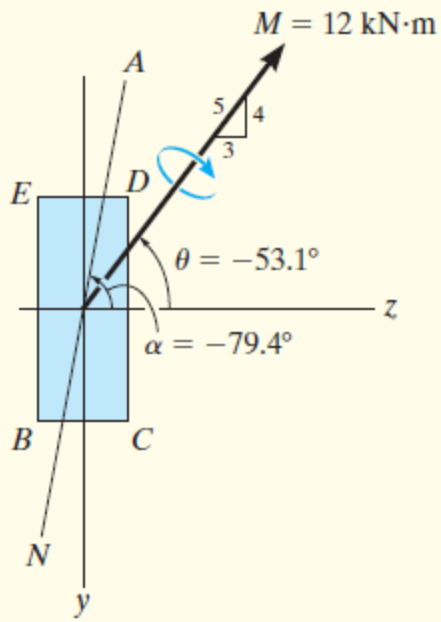
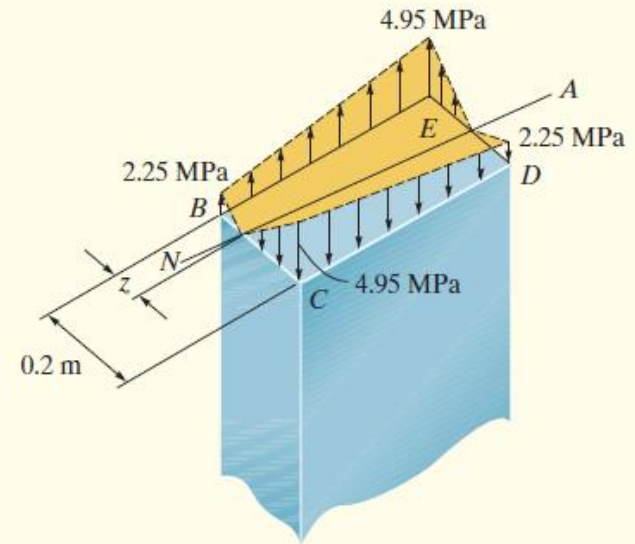
## Bending Stress.



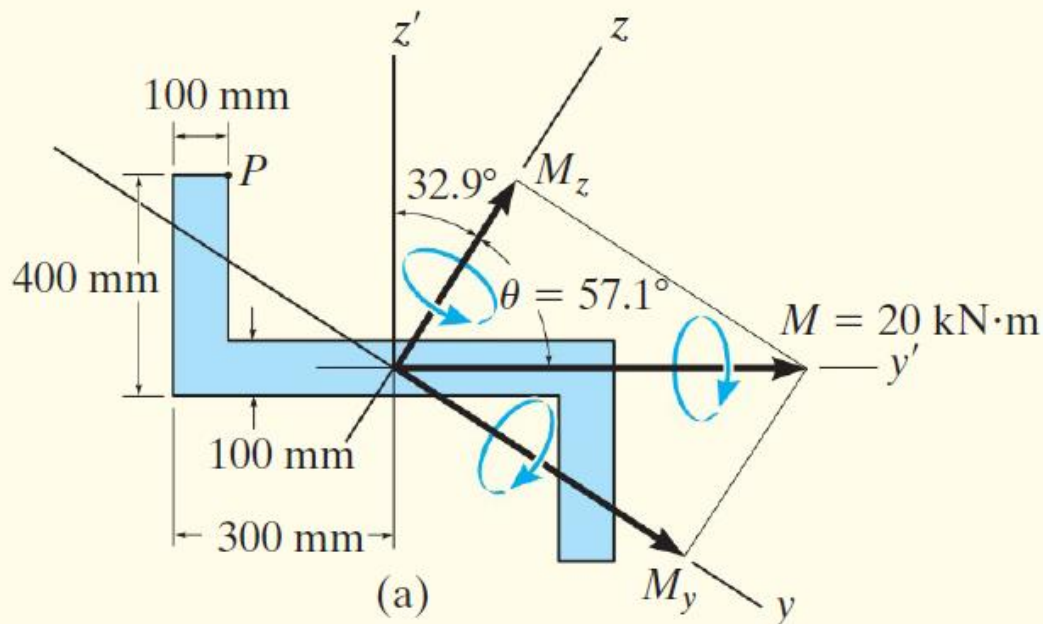
# BENDING

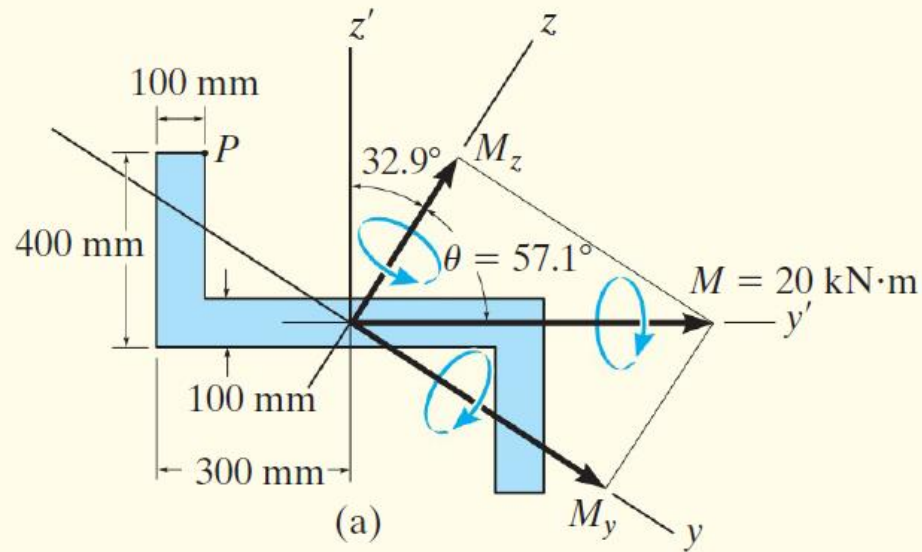
## فصل پنجم: خمشی

### Orientation of Neutral Axis.



The Z-section shown in Fig. 6–34a is subjected to the bending moment of  $M = 20 \text{ kN} \cdot \text{m}$ . The principal axes  $y$  and  $z$  are oriented as shown, such that they represent the minimum and maximum principal moments of inertia,  $I_y = 0.960(10^{-3}) \text{ m}^4$  and  $I_z = 7.54(10^{-3}) \text{ m}^4$ , respectively.\* Determine the normal stress at point  $P$  and the orientation of the neutral axis.



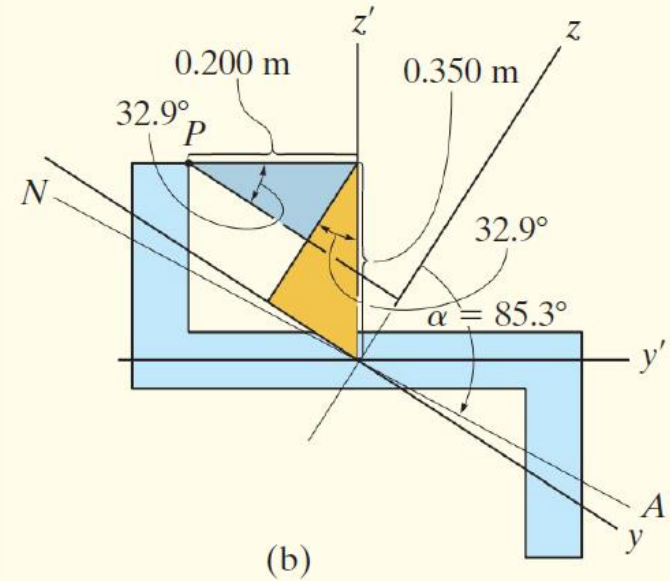


### Internal Moment Components.

$$M_y = 20 \text{ kN} \cdot \text{m} \sin 57.1^\circ = 16.79 \text{ kN} \cdot \text{m}$$

$$M_z = 20 \text{ kN} \cdot \text{m} \cos 57.1^\circ = 10.86 \text{ kN} \cdot \text{m}$$

## Bending Stress.



## Orientation of Neutral Axis.